

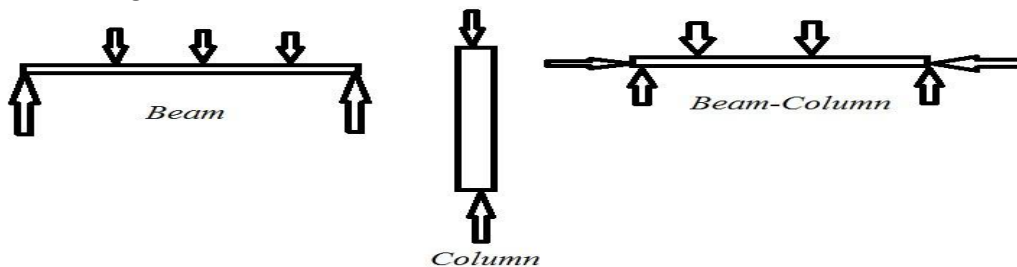
## UNIT - II

### Shear Force and Bending Moment

Definition of beam – Types of beams – Concept of shear force and bending moment – S.F and B.M diagrams for cantilever, simply supported and overhanging beams subjected to point loads, uniformly distributed load, uniformly varying loads and combination of these loads – Point of contraflexure – Relation between S.F., B.M and rate of loading at a section of a beam.

#### Definition: Beam

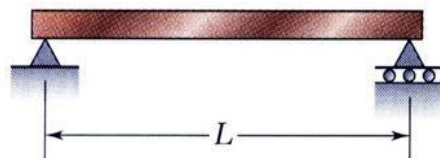
- A bar working under bending is generally termed as beam.
- A beam is laterally (Transverse) loaded member, whose cross-sectional dimensions are small as compared to its length.
- A beam may be defined as a structural member subjected to external loads at right angles to its longitudinal axis. If the external force acts along the longitudinal axis, it is called column. Material – Wood, Metal, Plastic, Concrete



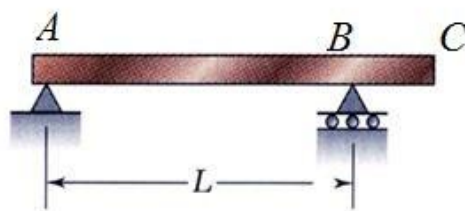
#### Types of beams: According to their support

1. **Simply Supported beam:** if their supports creates only the translational constraints.

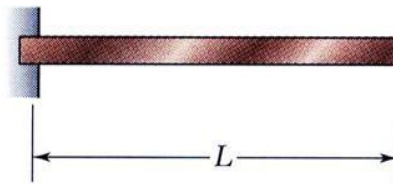
Sometime translational movement may be allowed in one direction with the help of rollers.



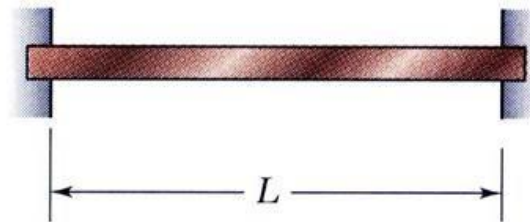
2. **Overhanging beam:** A beam which is simply supported at point A and B and projects beyond point B. The segment BC is similar to cantilever beam but also the beam axis may rotate at point B.



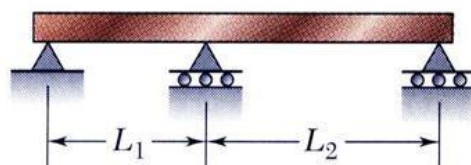
3. **Cantilever beam:** fixed at one end and free at other end. At fix support the beam can neither translate nor rotate, whereas at the free end it may do both. Therefore force & movement reactions may exist at the fixed support.



4. **Fixed beam:** When both end is fixed.



5. **Continuous beam:** More than two supports are there.



### Types of loading

- a. **Concentrated or point load:** When external load acting on the beam is concentrated at a single point on the beam.
- b. **Uniformly distributed load (UDL):** When external load acting on the beam is distributed over a length of beam, the following load is said to be a UDL. Ex-self-weight of beam, water pressure at bottom of water tank. It is represented as magnitude of load per unit length. For solving numerical problem, the total UDL is converted into point load, acting at the centre of UDL.
- c. **Uniformly varying load (UVL):** Load spread over a beam in such a manner that rate of loading varies uniformly from point to point. Also known as triangular load.

### Shear Force (SF)

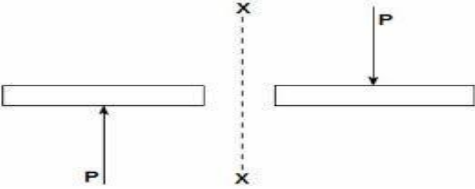
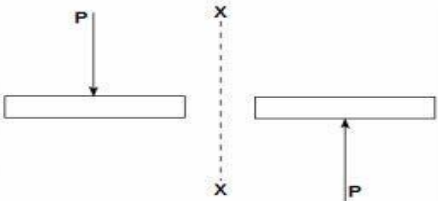
When the beam is loaded in some arbitrarily manner, the internal forces and moments are developed and the term shear force and bending moment come into pictures.

The algebraic sum of vertical forces at any section of beam to the right or left of section is known as Shear Force.

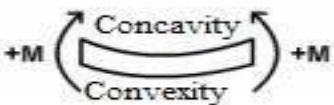
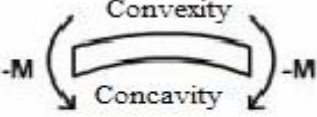
## Bending Moment (BM)

Algebraic sum of moments of all the forces acting to the right or left of the section is known as bending moment.

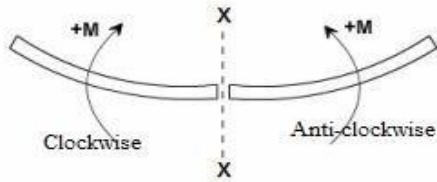
Sign Convention for Shear Force: Shear Force at section will be considered as

<b>Positive (+)</b>	<b>Negative (-)</b>
<p>When resultant of forces to the left to the section is upwards or right of the section is downwards.</p> 	<p>When the left hand portion tend to slide downwards or right hand portion tend to slide upwards</p> 

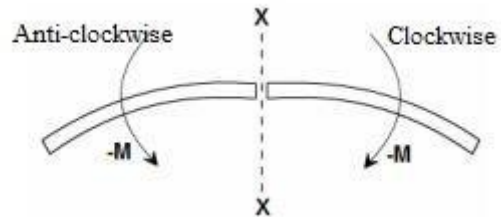
Sign Convention for Bending Moment:

<b>Positive (+)</b>	<b>Negative (-)</b>
<p>We take bending moment as a section as positive, if it tend to bend the beam at that point to a curvature having concavity at top or convexity at the bottom.</p>  <p style="text-align: center;"><b>Sagging</b></p>	<p>We take bending moment as a section as negative, if it tend to bend the beam at that point to a curvature having convexity at top or concavity at the bottom.</p>  <p style="text-align: center;"><b>Hogging</b></p>
Called Sagging Bending Moment	Called Hogging Bending Moment

When it is acting in clockwise direction to the left or anti-clockwise direction to the right.



When it is acting in anti-clockwise direction to the left or clockwise direction to the right.

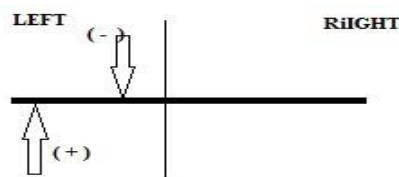
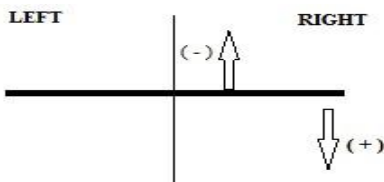


## Shear Force Diagrams and Bending Moment Diagrams

Before taking up the design of any structural element the structure is to be analysed and magnitude of bending moments and shear force should be determined. The structural element is designed for maximum bending moment and for maximum shear force. The BMD help to a great extent in identifying the tensile zones in reinforced concrete structure for providing steel reinforcement at appropriate place.

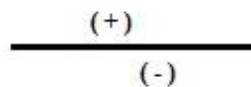
### Important points for drawing SFD & BMD:

1. Consider a left or right portion of section.
2. Add forces (including reactions) normal to the beam on one of the portion.  
If right portion of the section is chosen, a force on right portion acting downwards is positive, while force acting upwards is negative.

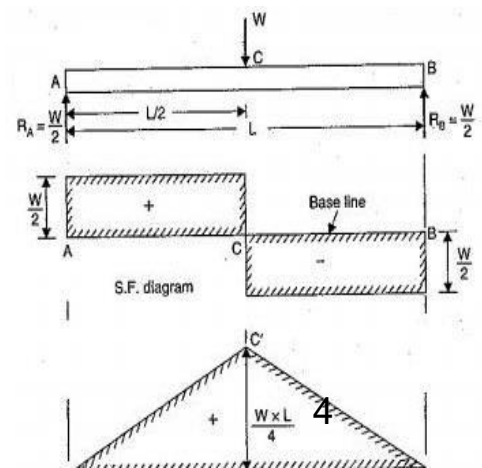


If left portion of the section is chosen, a force on left portion acting upwards is positive, while force acting downwards is negative.

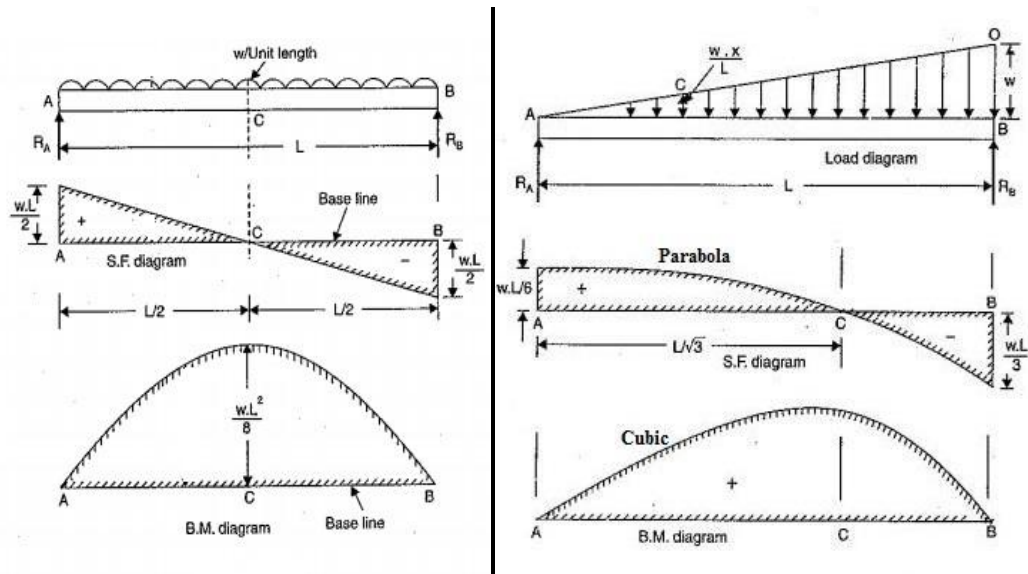
3. The positive value of SF and BM are plotted above the base line and negative value below the base line.



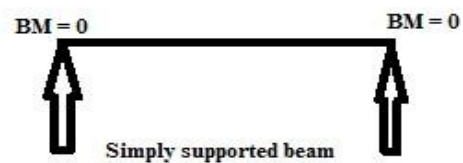
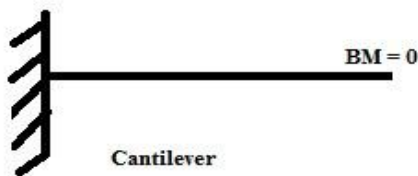
4. The SFD will increase or decrease suddenly i.e. by a vertical straight line at a section where there is a vertical point load but BM remains the same.
5. SF between any two vertical loads will be constant and hence SFD between two vertical loads will be horizontal.



6. If there is no load between two points, then the SF does not change (SF is horizontal) but BM changes linearly (inclined straight line).
7. If there is UDL between two points, then the SF changes linearly (SF is inclined by straight line) but BM changes according to parabolic law (BM will be parabola curve).



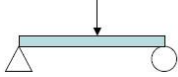
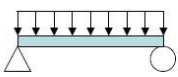
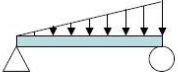
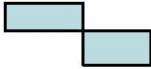

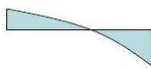
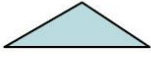


8. If there is UVL between two points, then the SF changes parabolic law (SF will be a parabola curve) but BM changes according to cubic law.
9. The BM at the two supports of a simply supported beam and the free end of cantilever will be zero.



10. Bending moment variation is 1 order a head than shear force variation.

Shear Force	Bending Moment
Rectangle or Constant	Linear or Triangle
Linear or Triangle	Parabola
Parabola	Cubic

## Common Relationships

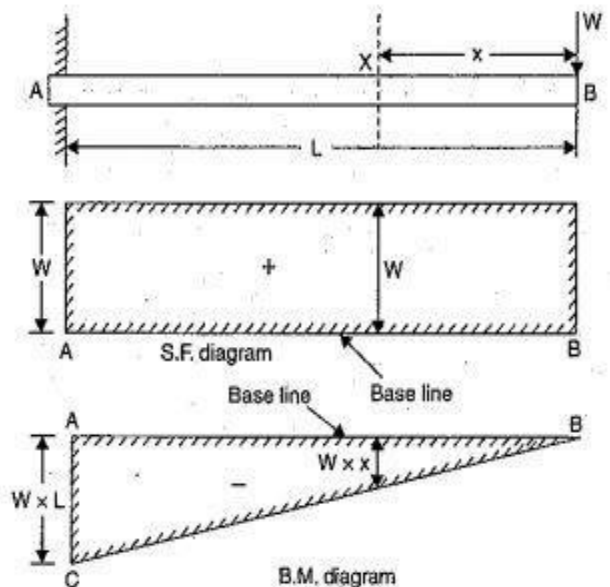
Load	0 	Constant 	Linear 
Shear	Constant 	Linear 	Parabolic 
Moment	Linear 	Parabolic 	Cubic 

### Cantilever beam with a point load at its free end

Consider a cantilever beam AB of length  $l$ , fixed at A and free at B, and carrying a point load  $W$  at the free end B.

#### Calculation for SFD:

- Take a section X-X at a distance of  $x$  from free end B.
- Consider a right portion of the section.
- The shear force at this section is equal to resultant force acting on the right portion at given section.
- But the resultant force acting on the right portion at the section X-X is  $W$  and acting in downward direction.
- Force on right portion acting downward is considered positive.
- Hence shear force at section X-X is positive.  
SF at section X-X =  $+W$
- There is no other load between A & B.  
So that Shear Force will be constant at all sections of cantilever beam.



#### Calculation for BMD:

Bending moment at section X-X =  $M_x = -W \cdot x$  ..... (i)

- Bending moment will be negative as for the right portion of the section, the moment of  $W$  at  $X-X$  is clockwise.
- Bending of cantilever will take place in such a manner that convexity will be at the top of the beam.
- From equation (i) it is clear that BM of a cantilever beam at any section is proportional to the distance of the section from the free end  
BM at point  $A_{(x=0)} = 0$

$$\text{BM at Point } B_{(x=l)} = -W.l$$

- Hence bending moment follows straight line for such cases.

**Q. A cantilever beam of length 2m carries the point loads as shown in figure. Draw shear force and bending moment diagram for cantilever beam.**

**Calculation for SFD:**

SF at point D = 800N

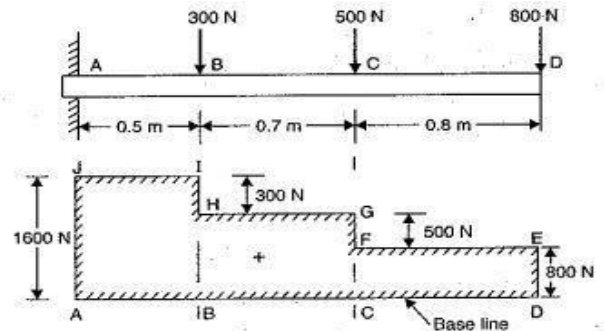
SF just right to C = 800N

Sf at point C =  $800+500 = 1300\text{N}$

SF just right to B = 1300N

SF at point B =  $1300+300 = 1600\text{N}$

Sf just right to A = 1600N



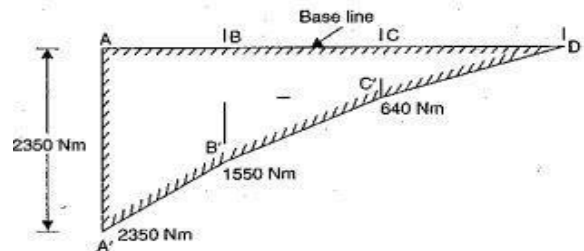
**Calculation for BMD:**

BM at point D = 0

BM at point C =  $-800 \times 0.8 = -640\text{N-m}$

BM at point B =  $-800 \times 1.5 - 500 \times 0.7$   
 $= -1550\text{N-m}$

BM at point A =  $-800 \times 2 - 500 \times 1.2 - 300 \times 0.5$   
 $= -2350\text{N-m}$



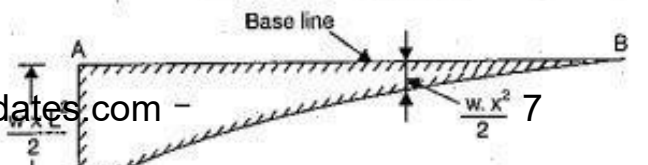
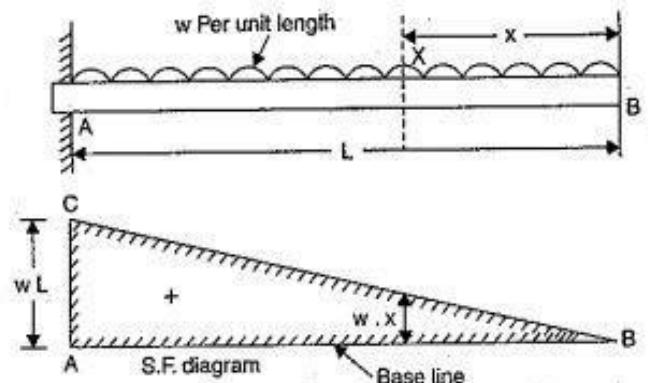
### Cantilever with Uniformly distributed load

Consider a beam AB of  $l$  length fixed at A and carrying a uniformly distributed load of  $w$  per unit length over the entire length of cantilever.

The SF at section  $X-X$  will be equal to the resultant force acting on the right portion of the section.

Resultant force on right portion =  $w.x$

The resultant force is acting downwards.



**Calculation for SFD:**

SF at section X-X =  $w \cdot x$

SF at point B<sub>(x=0)</sub> = 0

SF at point A<sub>(x=l)</sub> =  $w \cdot l$

**Calculation for BMD:**

As we discussed earlier that the UDL over a section is converted into point load acting at the C.G. of the section

BM at section X-X =  $-W \cdot x$

BM at Point B<sub>(x=0)</sub> = 0

BM at point A<sub>(x=l)</sub> =  $-W \cdot l$

**Q. A cantilever of length 2m carries a uniformly distributed load of 1kN/m run over a length of 1.5m from the free end. Draw the shear force and bending moment diagram for the cantilever.**

**Calculation for SFD:**

SF at section X-X between B & C at a distance of x from free end =  $w \cdot x$

SF at point B<sub>(x=0)</sub> = 0

SF at point C<sub>(x=1.5)</sub> = 1.5kN

SF just right to A = 1.5kN

**Calculation for BMD:**

BM at section X-X between B & C at a distance of x from free end =  $-w \cdot x$

BM at point B<sub>(x=0)</sub> = 0

BM at point C<sub>(x=1.5)</sub> =  $-1.125$  kN-m

BM at section Y-Y between A & C at a distance of y from free end

$$= -1 \times 1.5 \times \left[ \frac{1}{2} + (x-1.5) \right] = -1.5 (x-0.75)$$

[Total load due to UDL is =  $1 \times 1.5 = 1.5$  kN

This load will act at a distance =  $0.75$  m]

BM at point C<sub>(x=1.5)</sub> =  $-1.5 (1.5-0.75)$

$$= -1.125 \text{ kN-m}$$

BM at point A<sub>(x=2)</sub> =  $-1.5 (2-0.75)$

$$= -1.875 \text{ kN-m}$$

**Q. A cantilever of length 2m carries a UDL of 1.5kN/m run over the whole length and a point load of 2kN at a distance of 0.5m from the free end. Draw SF and BM diagram.**



**Calculation for SFD:**

SF at section X-X between B & C at a distance of x from free end =  $w \cdot x = 1.5x$

SF at point B ( $x=0$ ) = 0

SF just right to C ( $x=0.5$ ) =  $1.5 \times 0.5 = 0.75 \text{ kN}$

BM at any section Y-Y between A & C at a distance y from B (free end) is given by

BM at section Y-Y =  $(1.5y + 2) \text{ kN}$

SF at point C ( $y=0.5$ ) =  $0.75 + 2 = 2.75 \text{ kN}$

SF just right to A ( $y=2$ ) =  $5 \text{ kN}$

**Calculation for BMD:**

BM at section X-X between B & C at a distance of x from free end =  $-W \cdot X \cdot \frac{x}{2}$

$$= -1.5 \frac{x^2}{2} = -0.75x^2$$

BM at point B ( $x=0$ ) = 0

BM at point C ( $x=0.5$ ) =  $-0.75 \times 0.5^2$

$$= -0.1875 \text{ kN-m}$$

BM at section Y-Y between A & C at a distance of y from free end

$$= -1.5y \cdot \frac{y}{2} - 2(y - 0.5)$$

$$= -0.75y^2 - 2$$

( $y=0.5$ ) BM at point C ( $y=0.5$ )

$$= -0.75 \times 0.5^2$$

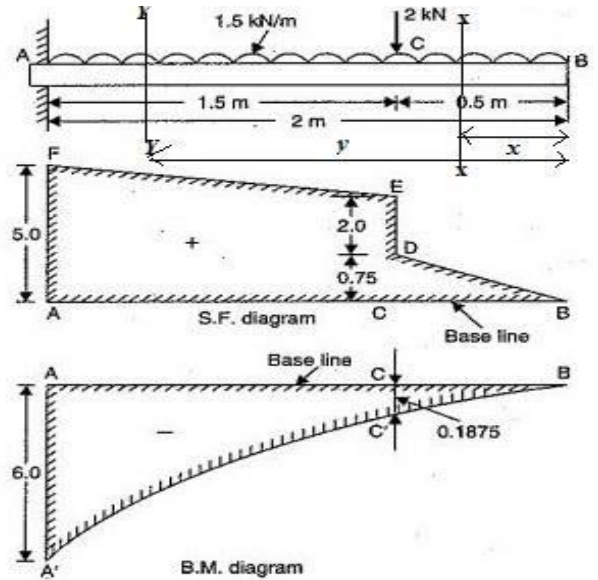
$$- 2(0.5 - 0.5)$$

$$=$$

$$- 0.1875 \text{ kN-m}$$

BM at point A ( $y=2$ ) =  $-0.75 \times 2^2 - 2(2 - 0.5)$

$$= -6 \text{ kN-m}$$



**Q. A cantilever 1.5m long is loaded with a uniformly distributed load of 2kN/m run over a length of 1.25m from the free end. It also carries a point load of 3kN at a distance of 0.25m from the free end. Draw SFD & BMD of cantilever.**

**Calculation for SFD:**

SF at point B = 0

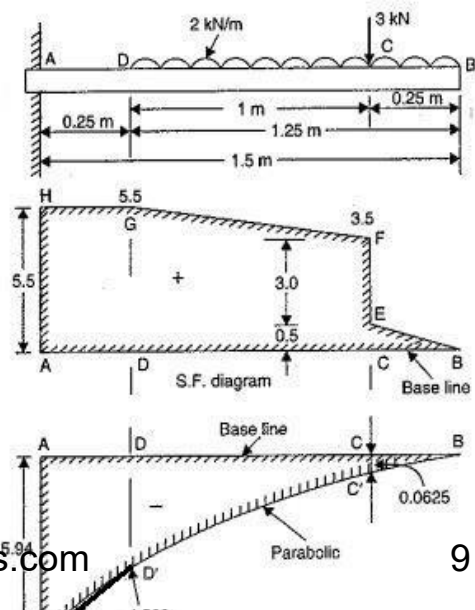
SF just right to C =  $2 \times 0.25 = 0.50 \text{ kN}$

SF at point C =  $0.50 + 3 = 3.50 \text{ kN}$

SF just right to D =  $3.5 + 2 \times 1 = 5.5 \text{ kN}$

SF at point D =  $5.5 \text{ kN}$

SF just right to A =  $5.5 \text{ kN}$



**Calculation for BMD:**

BM at Point B = 0

$$\text{BM at point C} = -2 \times 0.25 \times \frac{1}{2}$$

$$=$$

-0.0625 kN-m

BM at point D =

$$-2 \times 1.25 \times \frac{1}{2} - 3 \times 1$$

$$= -4.5625 \text{ kN-m}$$

$$\text{BM at point A} = -2 \times 1.25 \times [1 + 0.25] - 3 \times [1 + 0.25] = -5.9375 \text{ kN-m}$$

**Q. A cantilever of length of 5 m is loaded as shown in figure. Draw SFD and BMD for cantilever.**

**Calculation for SFD:**

SF at point B = 2.5 kN

SF just right to C = 2.5 kN

SF at point C = 2.5 kN

SF just right to D = 2.5 + 1 × 2 = 4.5 kN

SF at point D = 4.5 kN

SF just right to E = 4.5 kN

SF at point E = 4.5 + 3 = 7.5 kN

SF just right to A = 7.5 kN

**Calculation for BMD:**

BM at Point B = 0

BM at point C = -2.5 × 0.5 = -1.25 kN-m

$$\text{BM at point D} = -2.5 \times (2 + 0.5) - 1 \times 2 \times \frac{1}{2}$$

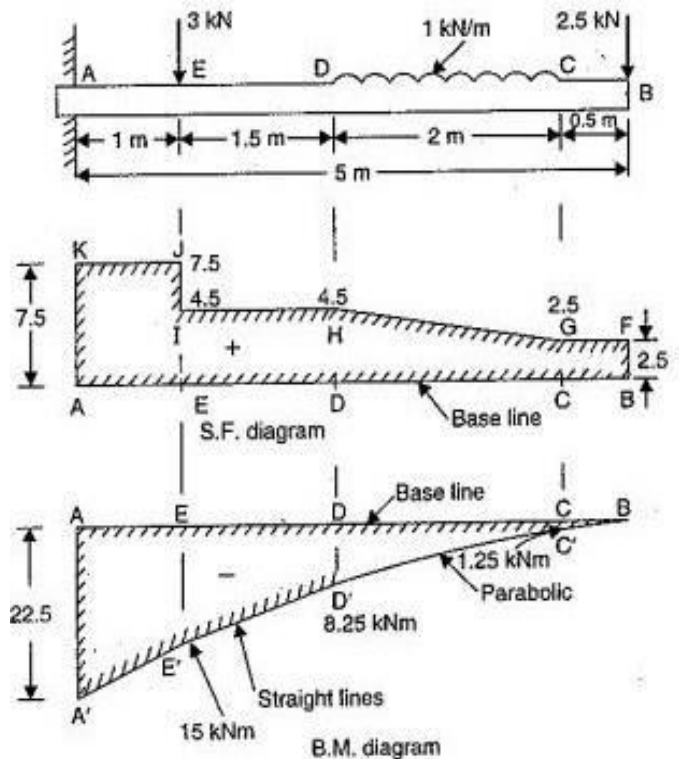
$$= -8.25 \text{ kN-m}$$

$$\text{BM at point E} = -2.5 \times [0.5 + 2 + 1.5] - 1 \times 2 \times [-1.5]$$

$$= 15 \text{ kN-m}$$

$$\text{BM at point A} = -2.5 \times 5 - 1 \times 2 \times [-2.5] - 3 \times 1$$

$$= -22.5 \text{ kN-m}$$



**Cantilever with a gradually varying load**

A cantilever of length  $l$  fixed at A and carries a gradually varying load from zero at free end to  $w$  per unit length at fixed end.

Rate of loading is 0 at B and is  $w$  per meter run at A

That means rate of loading for a length  $l$  is  $w$  per unit length

By using similar triangle ABE and CBD

— = — That means — = —

$$CD = \frac{x}{L} \times w$$

Rate of loading for a length of  $x = \frac{x}{L} \times w$  per unit length

**Calculation for SFD:**

SF at section X-X between A & B at a distance of  $x$  from free end = Total load on the cantilever for a length of  $x$  from the free end B

$$= \text{Area of triangle CBD} = \frac{1}{2} \times CB \times CD = \frac{1}{2} \times x \times \left(\frac{w \cdot x}{L}\right) = \frac{w \cdot x^2}{2L}$$

$$\text{SF at point B}(x=0) = 0$$

$$\text{SF at point A}(x=L) = \frac{w \cdot L^2}{2L} = \frac{w \cdot L}{2}$$

**Calculation for BMD:**

BM at section X-X between A & B at a distance of  $x$  from free end

$$= - (\text{Total load for a length } x) \times \text{Distance of load from C}$$

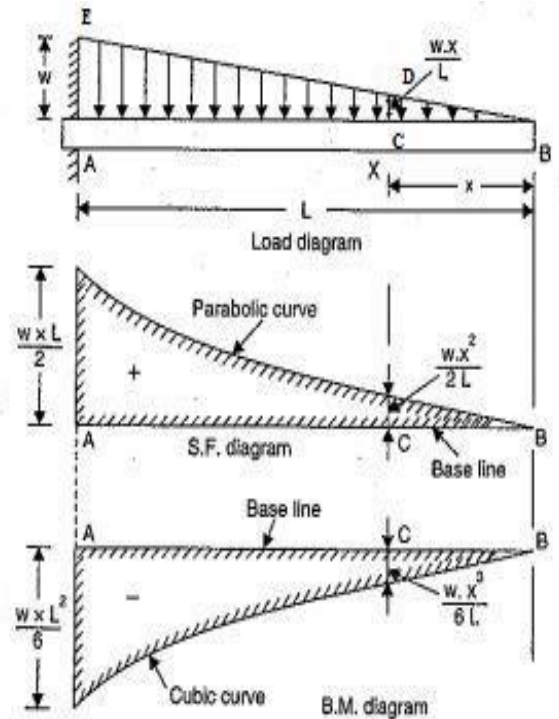
$$= - (\text{Area of Triangle CBD}) \times \text{Distance of C.G. of triangle from C}$$

$$= - \frac{w \cdot x^2}{2L} \times \frac{x}{3}$$

$$= - \frac{w \cdot x^3}{6L}$$

$$\text{SF at point B}(x=0) = 0$$

$$\text{SF at point A}(x=L) = - \frac{w \cdot L^3}{6L} = - \frac{w \cdot L^2}{6}$$



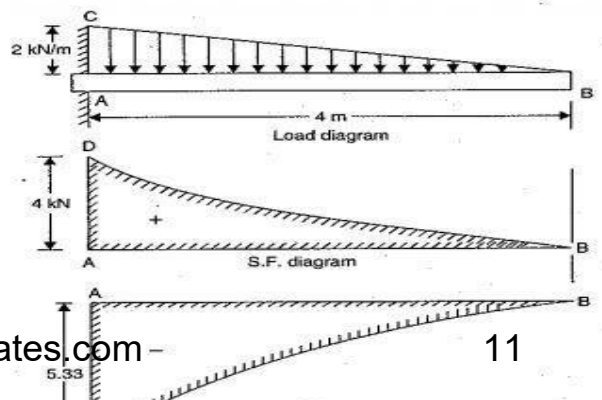
**Q. A cantilever of length 4m carries a gradually varying load, 0 at free end to 2kN/m at the fixed end. Draw SFD and BMD for cantilever.**

**Calculation for SFD:**

$$\text{SF at point B} = 0$$

$$\text{SF just right to A} = 4 \times \frac{2}{2} = 4 \text{ kN}$$

**Calculation for BMD: BM at Point B = 0**



BM at point C =  $-\frac{Wl}{4} = -5.33\text{kN-m}$

### Simply supported beam with a point load at its mid-point

A beam of length  $l$  simply supported at the ends A and B, carrying a point load  $W$  at its middle point C.

#### Calculation for Reaction:

$$R_A + R_B = W$$

$$\text{Taking moment at point A, } \Sigma M_A = 0$$

$$R_A \times 0 + R_B \times l = W \times \frac{l}{2}$$

$$R_B = \frac{W}{2}$$

$$R_A = \frac{W}{2}$$

#### Calculation for SFD:

$$\text{SF at point A} = \frac{W}{2}$$

$$\text{SF at point C} = \frac{W}{2} - W = -\frac{W}{2}$$

$$\text{SF just left to B} = -\frac{W}{2}$$

#### Calculation for BMD:

$$\text{BM at section X-X between A \& C at a distance of } x \text{ from its left end} = R_A \times x = \frac{W}{2} \times x$$

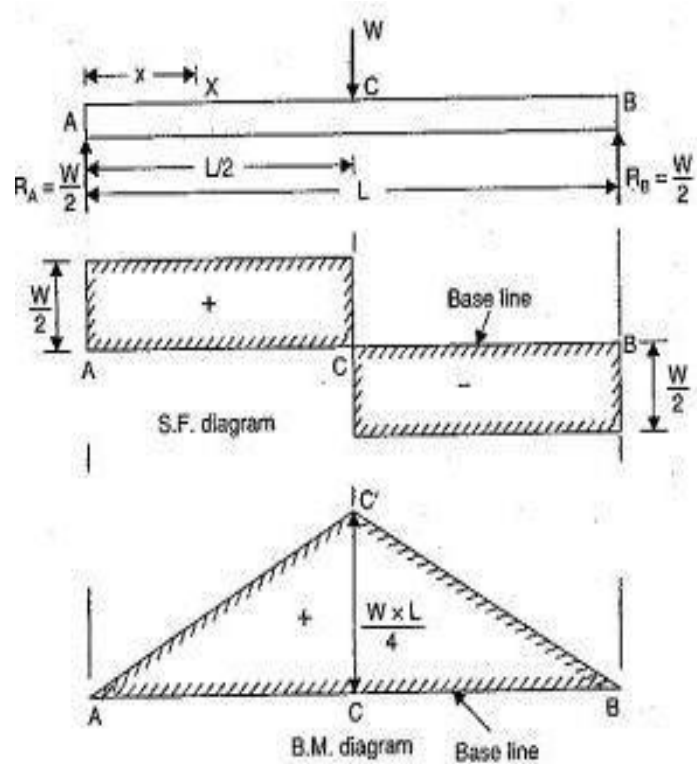
$$\text{BM at point A } (x=0) = 0$$

$$\text{BM at point B } (x=l) = \frac{W}{2} \times l - W \times \left(l - \frac{l}{2}\right) = 0$$

$$\text{BM at section Y-Y between B \& C at a distance of } y \text{ from its left end} = R_A \times Y - W(y - \frac{l}{2})$$

$$\text{BM at point C } (x=l/2) = \frac{W}{2} \times \frac{l}{2} - W \times \left(\frac{l}{2} - \frac{l}{2}\right) = \frac{Wl}{4}$$

$$\text{BM at point B } (x=l) = \frac{W}{2} \times l - W \times \left(l - \frac{l}{2}\right) = 0$$



### Simply supported beam with eccentric point load

#### Calculation for Reaction:

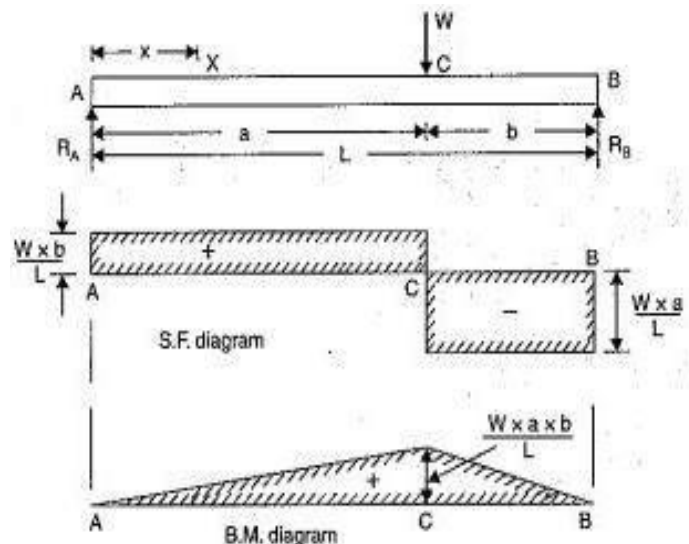
$$R_A + R_B = W$$

$$\Sigma M_A = 0$$

$$R_A \times 0 + l \times R_B = W \times a$$

$$R_B = \frac{W \times a}{l}$$

$$R_A = W - \frac{W \times a}{l} = \frac{W(l-a)}{l}$$



**Calculation for SFD:**

SF at point A = —

Sf at point C = — - W = - —

SF left to B = - —

**Calculation for BMD:**

BM at point A = 0

BM at point C = —

BM at point B = — - Wb = 0

**Q. A simply supported beam of length 6m carries point load of 3kN and 6kN at distance of 2m and 4m from the left end. Draw SFD and BMD for the beam.**

**Calculation for Reaction:**

$$R_A + R_B = 3 + 6 = 9$$

$$\Sigma M_A = 0$$

$$R_A \times 0 + R_B \times 6 = 3 \times 2 + 6 \times 4$$

$$6R_B = 30$$

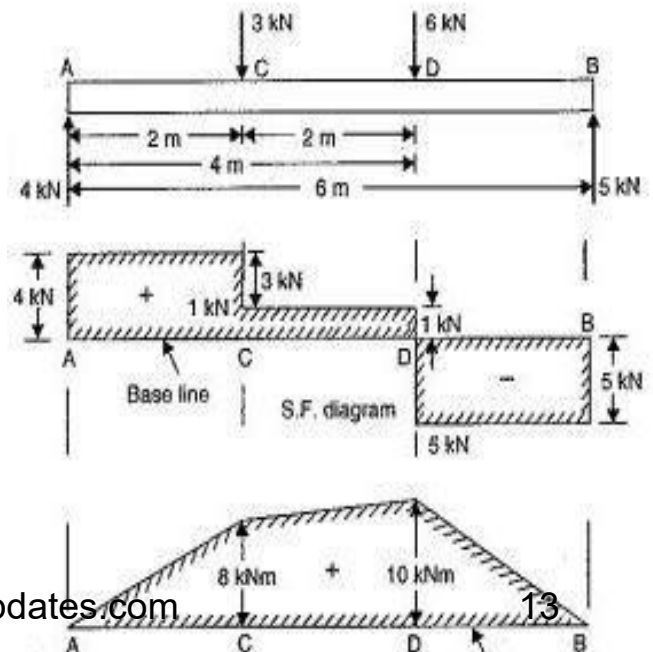
$$R_B = 5 \text{ kN}$$

$$R_A = 4 \text{ kN}$$

**Calculation for SFD:**

SF at point A = 4kN

SF at point C = 4 - 3 = 1kN



SF at point D =  $1 - 6 = -5\text{kN}$

SF just left to B =  $-5\text{kN}$

***Calculation for BMD:***

BM at point A = 0

BM at point C =  $4 \times 2 = 8\text{ kN-m}$

BM at point D =  $4 \times 4 - 3 \times 2 = 10\text{ kN-m}$

BM at point B =  $4 \times 6 - 3 \times 4 - 6 \times 2 = 0\text{kN-m}$

## Simply Supported beam with a uniformly distributed load

A beam AB of length  $l$  simply supported at the end A & B and carrying a uniformly distributed load of  $w$  per unit length over the entire length.

The reaction at their support will be equal and their magnitude will be half the load on the entire length.

### Calculation for Reaction:

$$R_A + R_B = w.l$$

$$\sum M_A = 0$$

$$R_A \times 0 + l \times R_B = w.l$$

$$R_B = \frac{w.l}{2}$$

$$R_A = w.l - \frac{w.l}{2} = \frac{w.l}{2}$$

### Calculation for Shear Force Diagram:

Shear Force at point A =  $\frac{w.l}{2}$

Shear Force just left to C =  $\frac{w.l}{2} - w \times \frac{l}{2} = 0$

Shear Force just left to B =  $-\frac{w.l}{2}$

### Calculation for Bending Moment Diagram:

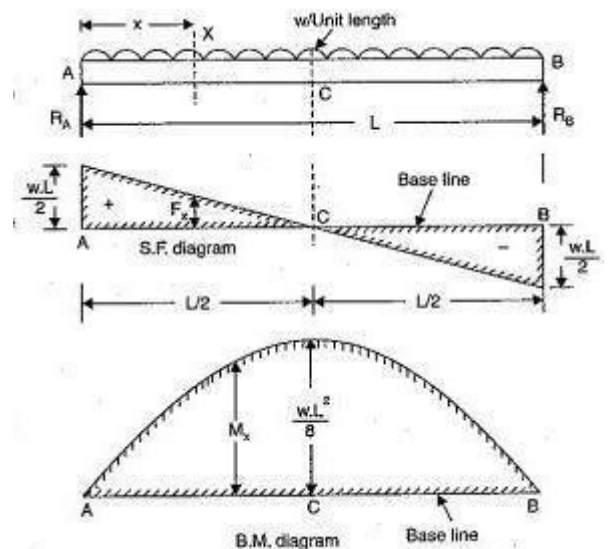
BM at point A =  $R_A \times 0 = 0$

BM at point C =  $R_A \times \frac{l}{2} - w \times \frac{l}{2} \times \frac{l}{4}$

$$= \frac{w.l}{2} \times \frac{l}{2} - \frac{w.l^2}{8}$$

$$= \frac{w.l^2}{8}$$

BM at point B =  $-\frac{w.l}{2} \times \frac{l}{2} + w \times \frac{l}{2} \times \frac{l}{4} = 0$



**Q. Draw Shear Force Diagram and Bending Moment Diagram for a simply supported beam of length 9m and carrying a uniformly distributed load of a 10kN/m for a distance of 4m as shown in figure.**

**Calculation for Reaction:**

$$R_A + R_B = 10 \times 4 = 40$$

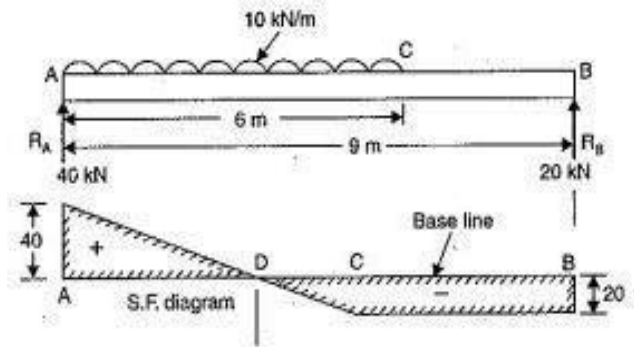
$$\sum M_A = 0$$

$$R_A \times 0 + 9 \times R_B = 10 \times 6 \times -$$

$$9 \times R_B = 180$$

$$R_B = 20 \text{ kN}$$

$$R_A = 40 \text{ kN}$$



**Calculation for Shear Force Diagram:**

Shear Force at point A = 40 kN

$$\text{Shear Force just left to C} = 40 - 10 \times 6 = -20 \text{ kN}$$

Shear Force at Point C = -20 kN

Shear Force just left to B = -20 kN

SF at A is +40 kN & at C = -20 kN

SF between A to C varies by a straight line

This means somewhere between A to C, the SF is Zero

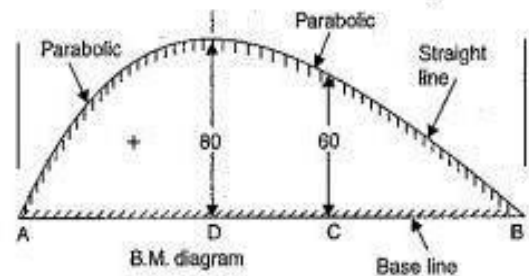
Let SF is Zero at a distance of x from A

Substitute SF value at this section equal to Zero

$$\text{Shear Force at section X-X between A \& C} = 40 - 10 \times x = 0$$

$$x = 4 \text{ m}$$

Hence SF will be Zero at a distance of x= 4m from A



**Calculation for Bending Moment Diagram:**

$$\text{BM at point A} = R_A \times 0 = 0$$

$$\text{BM at point C} = 40 \times 6 - 10 \times 6 \times -$$

$$= 240 - 180 = 60 \text{ kN-m}$$

$$\text{BM at point B} = 40 \times 9 - 10 \times 6 \times (- + 3) = 0$$

We know BM will be Maximum where SF changes its sign i.e. SF is Zero.

$$\text{Maximum Bending Moment} = 40 \times 4 - 10 \times 4 \times - = 80 \text{ kN-m}$$



**Q. Draw the Shear Force Diagram and Bending Moment Diagram for a simply supported beam of length 8m and carrying a uniformly distributed load of 10 kN/m for a distance of 4m as shown in figure.**

**Calculation for Reaction:**

$$R_A + R_B = 10 \times 4 = 40$$

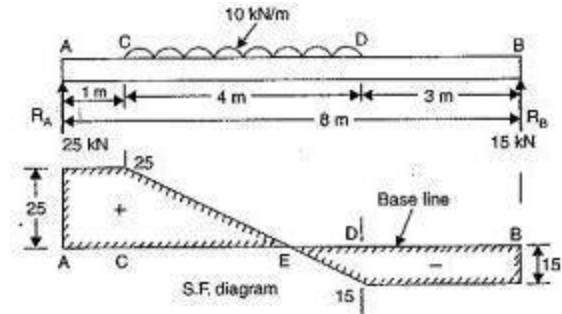
$$\Sigma M_A = 0$$

$$R_A \times 0 + 8 \times R_B = 10 \times 4 \times (-$$

$$8 \times R_B = 120$$

$$R_B = 15 \text{ kN}$$

$$R_A = 25 \text{ kN}$$



**Calculation for Shear Force Diagram:**

Shear Force at point A = 25 kN

Shear Force at Point C = 25 kN

Shear Force at point D = 25 - 10 × 4 = -15 kN

Shear Force just left to B = -15 kN

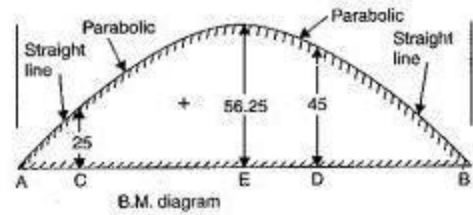
Shear Force at C = +25 kN and Shear Force at D = -15 kN

That means somewhere between C & D, Shear Force is Zero

Shear Force at section X-X between C & D = 25 - 10 × (x - 1) = 0

$$x = 3.5\text{m}$$

Shear Force at section X-X will be Zero.



**Calculation for Bending Moment Diagram:**

BM at point A =  $R_A \times 0 = 0$

BM at point C =  $25 \times 1 = 25 \text{ kN-m}$

BM at point D =  $25 \times 5 - 10 \times 4 \times \frac{1}{2} = 45 \text{ kN-m}$

BM at point B =  $25 \times 8 - 10 \times 4 \times (8 - 3) = 0 \text{ kN-m}$

Maximum Bending Moment (at  $x = 3.5\text{m}$ )-m =  $25 \times 3.5 - 10 \times 2.5 \times \frac{1}{2} = 56.25 \text{ kN-m}$

**Q. A simply supported beam of length 10m carries the uniformly distributed load and two points loads as shown in figure. Draw Shear Force Diagram and Bending Moment Diagram for the beam. Also Calculate maximum bending moment of the beam.**

**Calculation for Reaction:**

$$R_A + R_B = 50 + 40 + 10 \times 4 = 130 \text{ kN}$$

$$\Sigma M_A = 0$$

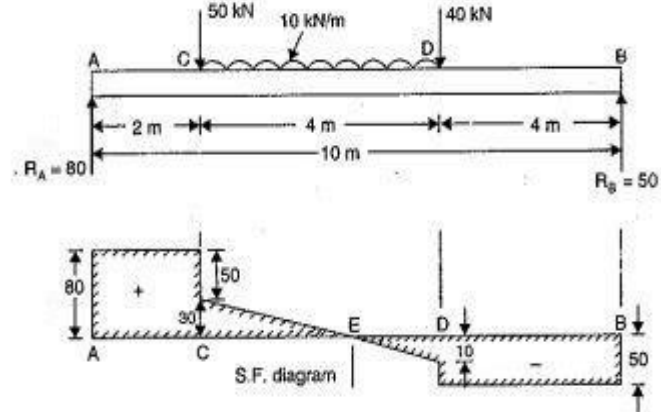
$$R_A \times 0 + 10 \times R_B = 50 \times 2 + 10 \times 4 \times (- + 2)$$

$$+ 40 \times 6$$

$$10 \times R_B = 100 + 240 + 160$$

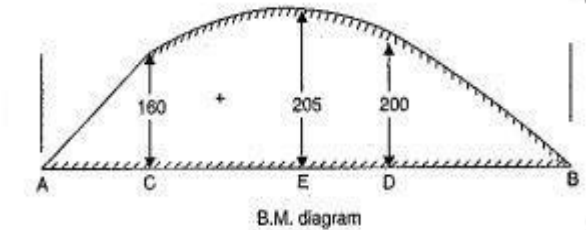
$$R_B = 50 \text{ kN}$$

$$R_A = 80 \text{ kN}$$



**Calculation for Shear Force Diagram:**

Shear Force at point A = 80 kN  
 Shear Force at Point C = 80 – 50 = 30 kN  
 Shear Force just left to D = 30 – 10 × 4 = -10 kN  
 Shear Force at point D = -10 – 40 = -50 kN



Shear Force just left to B = -50 kN  
 SF at section X-X between C & D = 80 – 50 – 10(x-2) = 0  
 30 – 10(x-2) = 0 ⇒ x = 5m

**Calculation for Bending Moment Diagram:**

BM at point A = RA × 0 = 0  
 BM at point C = 80 × 2 = 160 kN-m  
 BM at point D = 80 × 6 – 50 × 4 – 10 × 4 × - = 200 kN-m  
 BM at point B = 80 × 10 – 50 × 8 – 10 × 4 × (- + 4) – 40 × 4 = 0 kN-m

We know that Maximum bending moment will occur at that point where SF changes its sign

(SF = 0)

Maximum Bending Moment (at x = 5m) = 80 × 5 – 50 × 3 – 10 × 3 × - = 205 kN-m

**Simply supported beam carrying a uniformly varying load (Triangle load)**  
 (from zero at both ends to  $w$  per unit length at the centre )

Consider a simply supported beam AB of span  $l$  and carrying a triangular load, varying gradually from 0 at both ends to  $w$  per unit length at the centre.

Since the load is symmetrical therefore the reaction  $R_A$  &  $R_B$  will be equal.

Total load on beam = Area of load diagram

ABE

$$= - \times \text{base} \times \text{height} = - \times AB \times CE$$

$$= \frac{wl}{2}$$

Hence  $R_A = R_B =$  Half of the total load

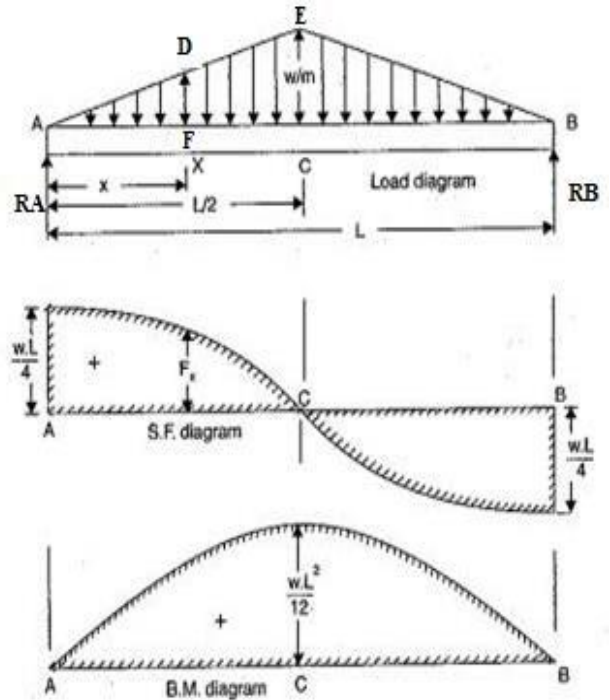
$$R_A = \frac{wl}{4} \quad \& \quad R_B = \frac{wl}{4}$$

Consider any section X-X between A & C at a distance  $x$  from A.

Rate of loading at section X-X = DF

$\Delta ACE$  &  $\Delta AFD$  is similar triangle, So

— — — — —



Load on length AF of the beam = Area of load diagram AFD

$$= - \times \text{base} \times \text{height} = - \times AF \times DF$$

$$= \frac{1}{2} \times x \times \frac{2wx}{l} = \frac{wx^2}{l}$$

This load will act at a distance of  $\frac{x}{3}$  from DF

**Calculation for SFD:**

SF at section X-X =  $R_A$  – load on length AF

— — —

{It showing parabolic equation}

SF at point A( $x=0$ ) = —

SF at point C(x=l) = - - - = 0

SF just left to B<sub>j</sub> = - - -

**Calculation for BMD:**

BM at section X-X = R<sub>A</sub> × x - load on length AF ×

$$= \frac{Wl}{4} \times x - \frac{wx^2}{l} \times \frac{x}{3}$$

$$= \frac{wx^3}{12} - \frac{wx^3}{6l}$$

{It showing cubic

equation}

BM at point A(x=0) = 0

BM at point C(x=l) = - ×  $\frac{l}{4}$  -  $\frac{w}{l} \times (\frac{l}{4})^3 = 0$

$$= \frac{wl^2}{8} - \frac{wl^2}{24} = \frac{wl^2}{12}$$

$$= \frac{wl^2}{12} \quad \text{{where W =$$

$$\frac{wl}{2} \text{}}$$

BM just left to B = 0

**NOTE** – Bending moment is always maximum, where Shear Force become zero after changing its sign

**NOTE** – In case of simply supported beam, bending moment will always be zero at both ends (support)

**Q. A simply supported beam of 5m span carries a triangular load of 30kN, Draw SFD and BMD for the beam.**

**Calculation for reaction:**

$R_A = R_B =$  Half of the total load

$R_A = R_B = 15$  kN

**Calculation for SFD:**

SF at point A = 15 kN

SF at point C =  $15 - (30/2) = 0$

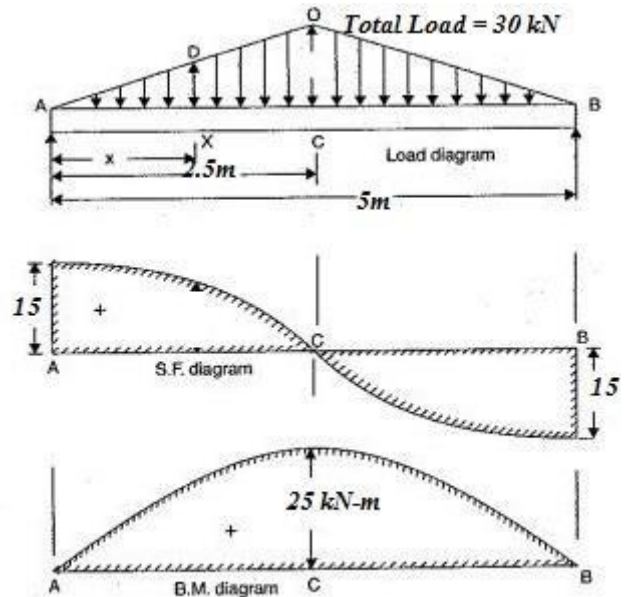
SF just left to B =  $- 15$  kN

**Calculation for BMD:**

BM at point A =  $R_A \times 0 = 0$

BM at point C =  $15 \times 2.5 - \frac{1}{2} \times 30 \times 2.5 = 25$  kN-m

BM at point B =  $15 \times 5 - \frac{1}{2} \times 30 \times 5 = 0$  kN-m



Simply Supported beam with a gradually varying load from zero at one end to  $w$  per unit length at other end

Consider a simply supported beam AB of length  $l$  and carrying a gradually varying load from 0 at A to  $w$  per unit length at B

**Calculation for reaction:**

Total load on beam = Area of triangle ABE

$$= \frac{1}{2} \times \text{base} \times \text{height}$$

$$= \frac{1}{2} \times w \times l$$

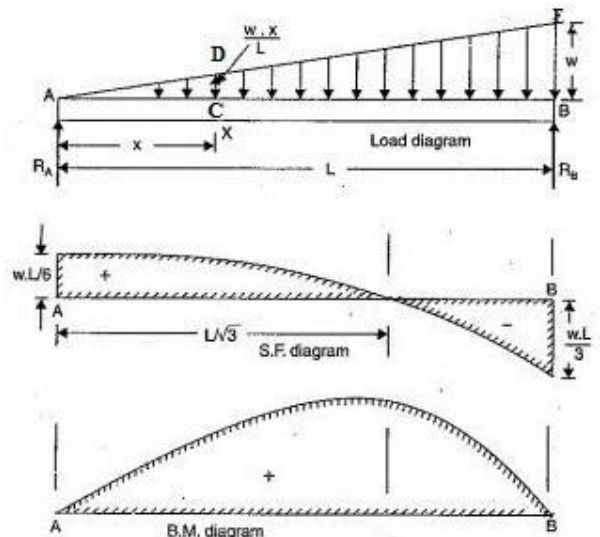
$$R_A + R_B = \frac{wL}{2}$$

$$\Sigma M_A = 0$$

$$R_A \times 0 + R_B \times L = \frac{wL}{2} \times \frac{L}{3}$$

$$R_B = \frac{wL}{6}$$

$$R_A = \frac{wL}{3}$$



Consider any section X-X between A & B at a distance  $x$  from A

Rate of loading at section X-X =  $DC$

$\Delta ABE$  and  $\Delta ACD$  are two similar triangles

— — — — —

**Calculation for SFD:**

SF at section X-X =  $R_A$  – load on length AC

$$= \frac{Wl}{6} - \frac{1}{2} \times x \times \frac{wx}{l}$$

{It showing parabolic equation}

$$= \frac{Wl}{6} - \frac{wx^2}{2l}$$

SF at point A( $x=0$ ) = —

SF just left to B = — - — = - —

We know that Maximum bending moment will occur at that point where SF changes its sign (SF = 0)

SF at section X-X between A & B at a distance of  $x$  from A

$$\frac{Wl}{6} - \frac{wx^2}{2l} = 0 \quad x = \frac{l}{\sqrt{3}}$$

Or  $x = 0.577 l$

**Calculation for BMD:**

BM at section X-X =  $R_A \times x$  – load on length AC $\times$

$$= \frac{Wl}{6} \times x - \frac{1}{2} \times x \times \frac{wx}{l} \times \frac{x}{3}$$

$$= \frac{Wlx}{6} - \frac{wx^3}{6l}$$

{It showing cubic equation}

BM at point A( $x=0$ ) = 0

BM at point C = — - — = 0

**Calculation for Maximum Bending Moment:**

BM will be maximum at that point where Shear Force is zero, i.e. at  $x = \frac{l}{\sqrt{3}}$

$$\text{Maximum bending moment (at } x = \frac{l}{\sqrt{3}}) = \frac{Wl}{6} \times \frac{l}{\sqrt{3}} - \frac{w}{6l} \times \left[\frac{l}{\sqrt{3}}\right]^3 = \frac{wl^2}{9\sqrt{3}}$$

**Q. A simply supported beam of length 5m carries a uniformly increasing load of 800N/m run at one end to 1600N/m run at other end. Draw the shear force and bending moment diagram for the beam, also calculate the position & magnitude of maximum bending moment.**

Weight may be assumed split into

- a. Uniformly distributed load of 800N/m over the entire span
- b. Uniformly gradually varying load of 0 at C & 800N/m at B

$$\text{Total UDL} = W_{UDL} = w \times l = 800 \times 5 = 4000\text{N}$$

$$\text{Total UVL} = W_{UVL} = \frac{1}{2} \times \text{height} \times \text{width} = \frac{1}{2} \times 5 \times 800 = 2000\text{N}$$

$$\text{Total load acting on beam} = W_{UDL} + W_{UVL} = 4000 + 2000 = 6000\text{N}$$

$\Delta CDE$  and  $\Delta CGH$  are two similar triangles

$$\frac{ED}{CD} = \frac{HG}{CB} \quad \frac{800}{5} = \frac{HG}{x}$$

$$HG = 160$$

**Calculation for reaction:**

$$R_A + R_B = 6000\text{N}$$

$$\Sigma M_A = 0$$

$$R_A \times 0 + 5 \times R_B = 800 \times 5 \times \frac{5}{2} + 5 \times \left( \frac{1}{2} \times 5 \times 800 \right)$$

$$800 \times \left( \frac{5}{2} \right)$$

$$5R_B = 10000 + 6666.66$$

$$6 \times R_B = 54$$

$$R_B = 3333.33\text{N}$$

$$R_A = 2666.67\text{N}$$

**Calculation for SFD:**

SF at section X-X between A & B at a distance of x from A

$$\begin{aligned} &= R_A - \text{load on length AF} \\ &= R_A - (\text{Area of rectangle} + \text{Area of } \Delta CGH) \\ &= 2666.67 - 800 \times x - \left( \frac{1}{2} \times x \times 160x \right) \\ &= 2666.67 - 800x - 80x^2 \end{aligned}$$

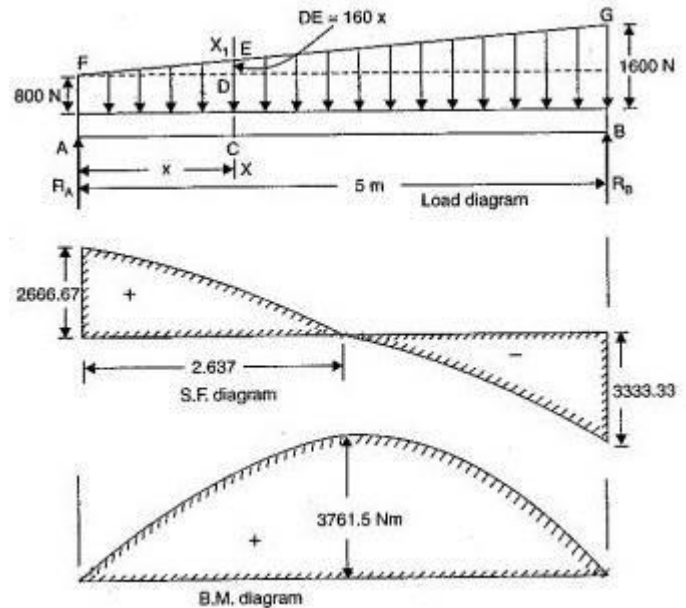
$$\text{SF at point A}_{(x=0)} = 2666.67\text{ N}$$

$$\text{SF at point B}_{(x=5)} = 2666.67 - 800 \times 5 - 80 \times 5^2 = -3333.33\text{N}$$

We know that, Shear Force changes its sign from positive to negative, that means at somewhere there will be a point where Shear will be Zero. And at this point Bending Moment will be maximum.

$$2666.67 - 800x - 80x^2 = 0$$

$$x = 2.637\text{m}$$



### Calculation for BMD:

BM at section X-X between A & B at a distance of  $x$  from A

$$\begin{aligned} &= R_A \times x - 800 \times x \times \frac{x}{2} - \left( \frac{1}{2} \times x \times 160x \right) \times \frac{x}{3} \\ &= 2666.67x - 400x^2 - \frac{80x^3}{3} \end{aligned}$$

BM at point A ( $x=0$ ) = 0

BM at point B ( $x=5$ ) =  $2666.67 \times 5 - 400 \times 5^2 - \frac{80 \times 5^3}{3}$  ———

Maximum Bending Moment ( $x=2.637$ ) =  $2666.67 \times 2.637 - 400 \times 2.637^2 - \frac{80 \times 2.637^3}{3}$  —————

Maximum Bending Moment ( $x=2.637$ ) = 3761.5 kN-m

### Point of Contraflexure (or Point of Inflexion):

Bending moment in cantilever was negative, whereas that in a simply supported beam is positive. It is thus obvious that in an overhanging beam, there will be a point where Bending moment will change sign from negative to positive or vice-versa, such a point, where the bending moment changes sign is known as Point of contraflexure or point of inflexion.

- The point of contraflexure is the point where the bending moment changes its sign from positive to negative or bending moment from Sagging to Hogging and vice-versa.
- At point of contraflexure bending moment is zero.
- It is to be noted that all the points where BM is zero are not necessary point of contraflexure.

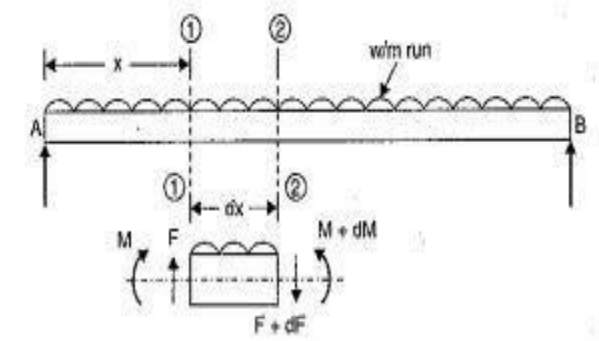
### Shear Force and Bending Moment Diagram for overhanging beams:

If the end portion of the beam is extended beyond the support, such beam is known as overhanging beam. Overhanging can be assumed as a combination of simply supported beam and cantilever beam. Due to this, overhanging beam experienced point of contraflexure.



# Relationship between Load, Shear Force and Bending Moment

Consider a simply supported beam carrying uniformly distributed load of  $w$  per unit length. An element  $dx$  at a distance of  $x$  from the left end A is considered.



An equilibrium of the portion of beam between section 1-1 and section 2-2

$F + dF$  = Shear Force at section 2-2

$M$  = Bending moment at section 1-1

$M + dM$  = Bending moment at section 2-2

Total load on length  $dx$  of beam =  $w \cdot dx$

For equilibrium  $\Sigma V = 0$

$$F - w \cdot dx = F + dF$$

$$- w \cdot dx = dF$$

$$\frac{dF}{dx} = -w$$

This shows that the rate of change of Shear Force is equal to load.

Taking moment at section 2-2 i.e. at point D

$$M + F \cdot dx - w \cdot dx \cdot \frac{dx}{2} = M + dM$$

Neglecting higher power of small quantities

$$F \cdot dx - dM = 0$$

$$F = \frac{dM}{dx}$$

This relationship shows that the rate of change of bending moment is equal to shear force.

$$- W = \frac{dM}{dx}$$

$$- W = \frac{dM}{dx} \cdot \frac{dx}{dx} = \frac{dM}{dx}$$

$$W = - \frac{dM}{dx}$$